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# **Numerical modelling of three-dimensional wave-current interactions in complex environment : application to Alderney Race**

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16 Modelling three-dimensional wave-current-turbulence interactions in extreme tidal environ-  
17 ments is still challenging and necessary for the development of the tidal industry, particularly  
18 for the dimensioning of tidal converters. Following this objective, we focus our study on the  
19 most energetic tidal site in Western Europe, the Alderney Race (France). Due to the strong  
20 tidal current at this location, wave-current interactions were poorly studied by the past and  
21 often neglected. We propose to assess how they impact the Alderney Race hydrodynamic by  
22 the use of numerical modelling and in-situ measurements. In this study, the following wave-  
23 current interactions were observed : i) Stokes drift effects inducing an increase/decrease in  
24 the current depending on the angle between waves and current, with a maximum influence  
25 near the surface, ii) wave enhancement of the bottom friction reducing the tidal current,  
26 iii) refraction of waves by the current, generating changes in waves directions, and iv) wave  
27 breaking ascribed to tidal current, increasing the turbulent mixing. A non-stationary time  
28 delay, varying within a same tidal cycle, was noted, which is reduced by including the local  
29 wind effects and by adjusting the bottom stress formulation. This study shows that wave-  
30 current interactions play a non-negligible role in Alderney Race although the strong tidal  
31 current and that they need to consider by the tidal industry.

# 1. Introduction

Marine renewable energies represent an alternative to fossil energies, which contribute to climate change. Ocean energy from tidal currents has a great potential throughout the world, because the currents are reliable and predictable and could be strong enough for industrial exploitation (e.g. Lynn 2013). In addition, the visual impact of tidal stream devices are limited in comparison to offshore wind farms or some wave energy converters. However, installation and maintenance of tidal converters are more complex than for other technologies due to the particular hydrodynamic conditions of tidal sites. Ocean tidal energy is considered economically feasible for water depths shallower than 50 meters and a flow velocity larger than 2.5 m/s (e.g. Lewis et al. 2015). A key point for the development of tidal energy is resource characterisation, which includes tidal site selection, possible modifications of the hydro-sedimentary environment induced by turbines and the impact of sediment transport on devices.

The most energetic tidal site in Western Europe is the Alderney Race, located in France, between La Hague Cape and Alderney Island, with tidal current reaching 5 m/s during spring tide (e.g. Bahaj and Myers 2004). Field measurements by velocity profilers were conducted in the past to estimate the hydrodynamic resources of the Alderney Race (e.g. Thiebault et al. 2019), but complex conditions generally led to loss or breakage of scientific devices making it very difficult to complete the measurements. Radio-oceanography, with High Frequency (HF) or/and Very High Frequency (VHF) and/or X-band radars, is a relevant option to obtain real-time spatialised measurements of flow velocity and ocean wave characteristics (e.g. Lopez et al. 2019). Numerical modelling is a useful alternative to estimate tidal resources. Because the circulation is primarily driven by astronomical tides, it can be computed with a barotropic model forced by tidal components at its open boundaries (Thiebot et al. 2015). The design of tidal energy converters, however, requires knowledge of the vertical structure of flow velocity in order to assess material fatigue issues and correct assessment of the energy production. Vertical profile depends on tide, as well as on ocean waves, marine turbulence and hydrodynamic interactions. A three-dimensional (3D) fully-coupled wave-current model with an accurate modelling of turbulent mixing is therefore required.

Most of sites that are suitable for tidal converters, including Alderney Race in Normandy (France) and Fromveur in Brittany (France), are influenced by surface waves, that modify the vertical shear of ocean velocity. Major modifications occur near the surface and up to a depth of about one half wavelength, but also near the bottom mainly within the wave and current bottom boundary layers (e.g. Nielsen 1992). Near-surface, ocean velocity may be reduced or accelerated depending on the angle between wave direction and tidal current due to Stokes drift effects (e.g. Kemp and Simons 1983, 1982; Groeneweg and Klopman 1998). Ocean waves also change the vertical shear of the turbulent quantities because of wave-enhancement of

turbulence in the bottom boundary layer and near the surface (e.g Grant and Madsen 1979; Burchard 2001). Grant and Madsen (1979) proposed a time-invariant two-layer turbulent model to take into account the wave effects on the turbulence level near and beyond the bottom. Following the same idea, many studies have proposed different formulations for the time-invariant turbulent eddy viscosity (e.g. Christoffersen and Jonsson 1985; Sleath 1991). In the upper ocean, changes in turbulence levels due to waves are mainly caused by wave breaking and Langmuirs circulations (e.g. Agrawal et al. 1992; Craik and Leibovich 1976).

Lewis et al. (2017) and Thiebault and Sentchev (2017) explain that the vertical shear of the ocean velocity in tidal areas follows a power law in some cases. However, Lewis et al. (2017) highlight high variability in vertical shear, showing the necessity to improve our understanding of the hydrodynamic processes that cause this variability. Togneri et al. (2017) explain that the well-known turbulent closure  $k - \epsilon$  without modifications to include wave effects fails to reproduce the vertical structure of turbulent quantities. They observe an underestimate of turbulent kinetic energy while turbulent dissipation is overestimated. Guillou et al. (2016), Lewis et al. (2014) and Hashemi et al. (2015) have studied the influence of surface waves on the tidal energy estimate. On the whole, they found 10 – 20% variation due to waves, depending on the angle between the tidal current and surface waves. However, these earlier studies are idealised : Guillou et al. (2016) used three-dimensional radiation stresses that are constant over the depth, because they were in shallow waters. In addition, in the latter study, the real case of the Iroise Sea is treated but vertical shear of the ocean flow is not discussed. Lewis et al. (2014) employ the COASWT model (Warner et al. 2010) with three-dimensional radiation stresses of Mellor (2015), which are debated by Ardhuin et al. (2017) and Mellor (2017), and study an idealised case of a 3D wave-induced flow propagating over a seamount. Hashemi et al. (2015) simulate the real case of the tidal site off the north-western coast of Anglesey Island (Wales, UK), with the inclusion of wave effects, but these simulations are two-dimensional (depth-integrated). Therefore, 3D effects were not taken into account.

Ocean waves also influence the bottom friction because they modify the turbulence level near the bottom, particularly inside the wave bottom boundary layer. Grant and Madsen (1979) have conceptualised these processes by a large apparent roughness. Many laboratory and in-situ measurements (e.g Mathisen and Madsen 1996b,a) have supported this concept. Mathisen and Madsen (1999) added the streaming effects to the original form of the apparent roughness model established by Grant and Madsen (1979). Parameterised approaches based on the outputs of these studies have also been developed to formulate the bottom shear stress under waves and current action (e.g. Soulsby et al. 1993; Holmedal et al. 2000) and are widely used by the scientific community when numerical models are not able to explicitly resolve these interactions.

We propose to extend the existing studies by performing realistic 3D simulations with a fully-coupled wave-current model (Bennis et al. 2011, 2014, 2016, 2018) in order to understand, how ocean waves and tidal current interact in Alderney Race. The data and methods are described in Section 2 as follows : 2a. Study site and in-situ data, 2b. Numerical modelling, 2c. Details on coupling procedure and set-up, and 2d. Description of the numerical experiments. Results are shown and discussed in Section 3 which is divided into four parts : 3a. Tidal elevation, 3b. Sea states, 3c. Time series of the tidal stream velocity, and 3d. Vertical structure of the tidal stream velocity. Conclusions are drawn in Section 4.

## 2. Data and Methods

### *a. Study Site and Data Collection*

Alderney Race is located inside the English Channel (hereinafter EC) between the Alderney Island and La Hague Cape along the French coast, with a depth of 25-65 m (see Figure 1a). Due to the proximity of the Cherbourg harbour and its facilities, that facilitates marine operations, companies are interested in installing of marine currents turbines (MCTs) to produce electricity from tidal current. Alderney Race is a mega-tidal environment (e.g. Dauvin 2015), with a mean spring tidal range varying from 6 to 11 m from the north to the south of La Hague Cape (about 5 km between Anse de Saint Martin :  $49^{\circ}42'30''$  N/ $1^{\circ}53'0''$  W and Herqueville :  $49^{\circ}40'06''$  N/ $1^{\circ}52'34''$  W) and with a strong tidal asymmetry due to the interactions between tidal flow and bathymetry (see Figure 1b). The particular geometry of the Alderney Race, with the short distance, around 12 kms, between Alderney Island and La Hague Cape, generates a channel effect that accelerates the tidal flow up to 5 m/s during spring tides. The maximum mean potential power is estimated to be 5.1 GW (Coles et al. 2017). For comparison, this represents half of the French tidal resource (Bahaj and Myers 2004) and is 35% higher than the potential power of Pentland Firth, the best tidal site in United Kingdom.

Swells from the Atlantic Ocean propagate through the EC, mainly in the western part because they are often stopped by the Cotentin peninsula. Alderney Race, located west of this peninsula, though protected by the Alderney Island, is influenced by swells (e.g. Lopez et al. 2018). The dominant winds in La Hague Cape are south-west or west, with wind velocity stronger than 16 m/s about 130 days per year according to the French Weather Service (Météo-France). Thus, Alderney Race sea states are often complex, with superposition of swells and wind-seas. Maisondieu (2016) performed statistical analyses based on the HOMERE database (Boudiere et al. 2013) for the period between 2003 and 2012. The results were : i) about 40% of sea states had at least 3 swells, ii) about 30% of sea states had at least 2 swells and 1 wind-sea, and iii) about 20 % of sea states have at least 1 swell and 1

wind-sea. Furthermore, a chaotic sea was observed when the tidal current and wind directions were opposite, with wave heights of about 4 m and wavelengths shorter than 50 metres. Complex sea states also occurred and were recorded during the HYD2M experiments, with significant wave heights of about 8 m. Wave breaking is often observed in Alderney Race due to the interactions between waves and the tidal current, leading to the French name 'Raz Blanchard' (In English : 'White Race', named for the frequent white caps in this area). High energy marine turbulent structures are present in Alderney Race because of the very rough nature of the seabed, which leads to the ejection of turbulent cells from the bottom to the surface (Mercier 2019). These structures, a few tens of meters in length, are 3D and visible to the naked eye. They interact with the tidal current and ocean waves. Moreover, the bathymetry is very uneven with features and faults acting as several metre height barriers to the flow (Furgerot et al. 2019). The bottom sedimentology is strongly heterogeneous with sand, pebbles and large rocks (e.g. Larsonneur et al. 1982; Foveau et al. 2017; Furgerot et al. 2019).

ADCP data were collected by the HYD2M consortium (ADCP) in 2017 (see Figure 1a, yellow cross), using a bottom-mounted 500 kHz Teledyne RDI Sentinel V50. ADCP was located at  $49^{\circ}40'50.00''\text{N}/2^{\circ}01'46.44''\text{W}$ . The estimated mean depth was about 35 m. The bin size was 1 m and the lowest cell was 2 m above the seabed. ADCP data were collected from 14 October 2017 to 26 February 2018, but only days in the period of 21 to 25 November 2017 are considered here. ADCP recorded ocean wave characteristics in addition to measurements of vertical profile of the three components of the flow velocity. All ADCP data were 15 min-averaged. This means that high frequency variations, particularly due to turbulence, were not taken into account in this study, but were presented in Furgerot et al. (2018). Data from Met-Office wave buoys (62103 and 62027), available on EMODnet platform (<http://www.emodnet.eu>), were also used to validate the wave model, but comparison plots are not shown here. The simulated mean sea level was tested against measurements of Shom tidal gauges installed in Cherbourg (TG1, recordings from 1943 to now) and Di  lette (TG2, recordings from 2015 to now). Data are downloadable via the datahom portal (<https://data.shom.fr>). Tidal gauge locations are marked in black on Figure 1a. Wind data were collected by Gourey Semaphore, that is located 7 km apart ADCP point, at 10 meters above ground level.

The studied time period is representative of typical conditions in Alderney Race, except for extreme events. The met-oceanic conditions were : i) a tidal range between 4 and 7 m, ii) a tidal current varying from 0.2 m/s to 3 m/s, iii) a significant wave height ranging from 0.5 m - 4.5 m (with wind-waves and swells), and iv) a wind speed less than 18 m/s.

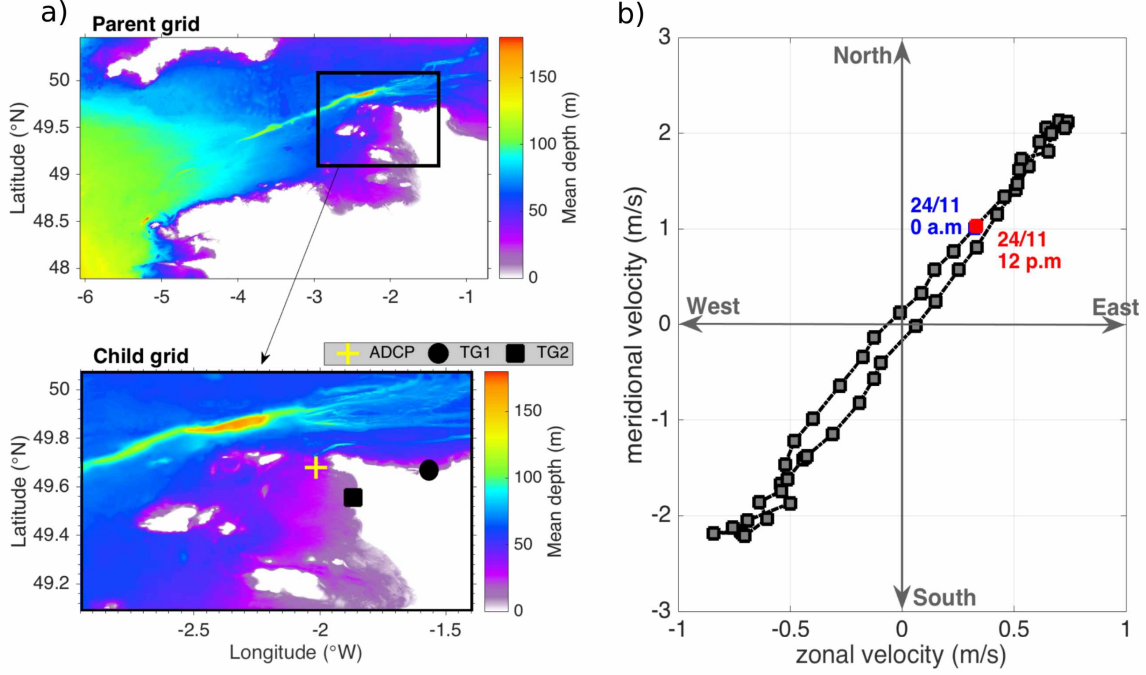


FIGURE 1: (a) ADCP (yellow cross) and tidal gauges (TG1 and TG2 in black circle and square, respectively) over the mean depth (colour scale). (b) Current hodograph for a 12-hour time period on 24 November 2017 between 0 a.m. (blue square) and 12 p.m. (red square).

### b. Numerical Modelling Strategy

Our modelling system couples a 3D ocean model, MARS3D v10 (Lazure and Dumas 2008), and the spectral wave model, WAVEWATCH III v4.08 (hereinafter WW3, Tolman and al. 2014). The wave-driven circulation is computed according to Ardhuin et al. (2008b) and Bennis et al. (2011). Wave forcing is based on the vortex force method which has been mainly validated for surf zone and also at coastal scales (e.g. Michaud et al. 2012; Moghimi et al. 2013; Bennis et al. 2014; Delpey et al. 2014; Bennis et al. 2016). This method considers the mean flow, represented by the quasi-Eulerian velocity (ie. the Lagrangian velocity minus the Stokes drift), rather than the total momentum, which removes the tricky problem of modelling the vertical flux of momentum (Ardhuin et al. 2008a). The generic formulation of momentum equations for a wave-forced, three-dimensional, incompressible, unsteady, hydrostatic, constant-density flow is :

$$\frac{D\hat{\mathcal{U}}}{Dt} = \mathbf{S}_{\text{EPG}} + \mathbf{S}_{\text{VM}} + \mathbf{S}_{\text{HM}} + \mathbf{S}_{\text{WP}} + \mathbf{S}_{\text{BA}} + \mathbf{S}_{\text{BBL}} + \mathbf{S}_{\text{VF}}, \quad (1)$$

where  $\hat{\mathcal{U}} = (\hat{U}, \hat{V}, \hat{W})$  is the 3D quasi-Eulerian velocity. The source terms  $\mathbf{S}_{\text{EPG}}$ ,  $\mathbf{S}_{\text{VM}}$ ,



191  $\mathbf{S}_{\text{HM}}$ ,  $\mathbf{S}_{\text{BA}}$ ,  $\mathbf{S}_{\text{BBL}}$ ,  $\mathbf{S}_{\text{VF}}$ ,  $\mathbf{S}_{\text{WP}}$  are related to the external pressure gradient, the vertical  
 192 mixing, the horizontal mixing, the breaking acceleration, the streaming, the vortex force and  
 193 the wave-induced pressure gradient, respectively. Wave-induced forcing terms are mainly the  
 194 vortex force, the Bernoulli Head, the forces induced by the wave-to-ocean momentum flux,  
 195 the wave-induced mixing and the wave-bottom interactions when the wave bottom boundary  
 196 layer is solved. These terms influence source terms of (1) (more details in Bennis et al. 2011).  
 197 This set of equations is compatible with that of McWilliams et al. (2004) used in Uchiyama  
 198 et al. (2010) and Kumar et al. (2012).

199 Horizontal mixing is grid-spacing dependent as in Smagorinsky (1963) with horizontal  
 200 viscosity ( $\nu_H$ ) defined as  $\nu_H = f_{\text{visc}} \cdot 0.01 \cdot (\Delta_{xy})^{1.15}$ , where  $\Delta_{xy}$  is the horizontal grid spacing  
 201 and  $f_{\text{visc}}$  is a user defined parameter (Okubo 1971).

202 The well-known k- $\epsilon$  turbulent scheme, modified according to Walstra et al. (2000) to  
 203 include ocean wave effects, is used for the vertical mixing :

$$\frac{\partial k}{\partial t} = \frac{1}{D^2} \cdot \frac{\partial}{\partial \zeta} \left( \frac{\nu_V}{s_k} \cdot \frac{\partial k}{\partial \zeta} \right) - \frac{\partial k}{\partial \zeta} \cdot \frac{\partial \zeta}{\partial t} + \text{Prod} + \text{Buoy} - \epsilon + \mathcal{P}_k, \quad (2)$$

$$\frac{\partial \epsilon}{\partial t} = \frac{1}{D^2} \cdot \frac{\partial}{\partial \zeta} \left( \frac{\nu_V}{s_\epsilon} \cdot \frac{\partial \epsilon}{\partial \zeta} \right) - \frac{\partial \epsilon}{\partial \zeta} \cdot \frac{\partial \zeta}{\partial t} + \frac{\epsilon}{k} (c_1 \text{Prod} + c_3 \text{Buoy}) + \mathcal{P}_\epsilon. \quad (3)$$

204 where  $k$  is the turbulent kinetic energy and  $\epsilon$  is the turbulent dissipation.  $\nu_V$  is the vertical  
 205 viscosity and depends on both mixing length and turbulent kinetic energy. Coefficients  $c_1$ ,  
 206  $c_3$ ,  $s_k$  and  $s_\epsilon$  are set according to Warner et al. (2005). Prod and Buoy terms represent the  
 207 turbulent production by shear and buoyancy, respectively. Equations (2) and (3) differ from  
 208 the classic ones : two source terms ( $\mathcal{P}_k$  and  $\mathcal{P}_\epsilon$ ) were added to include the mixing effects  
 209 relating to the bottom friction and wave breaking. At the surface, we preferred to use the  
 210 Dirichlet boundary conditions of Kantha and Clayson (2004), because they are based on  
 211 friction velocity, rather than the conditions of Walstra et al. (2000). Turbulent source terms  
 212 depend on wave energy dissipated by bottom friction and wave breaking, near-bottom wave  
 213 orbital velocity and wave bottom boundary layer thickness. They are linearly distributed  
 214 over a characteristic depth, that is equal to the root mean square significant wave height  
 215 divided by two near the surface and to the bottom boundary layer thickness near the bed  
 216 (more details in Walstra et al. 2000). While other distributions, e.g. trigonometric functions,  
 217 have been tested, only marginal differences have been noted.

Bottom friction and its enhancement by surface waves is parameterised with the formu-  
 lation of Soulsby (1995), such that the bottom stress ( $\tau_b$ ) is :

$$\tau_b = |\mathcal{T}_c| \cdot \left[ 1 + 1.2 \left( \frac{|\mathcal{T}_w|}{|\mathcal{T}_w| + |\mathcal{T}_c|} \right)^{3.2} \right], \quad (4)$$

where  $|\mathcal{T}_w|$  and  $|\mathcal{T}_c|$  are the shear stresses related to waves and current dynamic, such that :

$$|\mathcal{T}_c| = \rho \left[ \frac{\kappa}{\ln\left(\frac{z_m}{z_0}\right)} \right]^2 \cdot |\mathbf{u}_b|^2, \quad (5)$$

and

$$|\mathcal{T}_w| = \frac{1}{2} \rho f_w |\mathbf{u}_{orb}|^2. \quad (6)$$

where  $z_0$  is the bottom roughness,  $\rho$  is water mass density,  $\mathbf{u}_b$  and  $\mathbf{u}_{orb}$  are the nearbed ocean velocity and wave orbital velocity, respectively, and  $\kappa$  is Von-Karman's constant (set to 0.4),  $f_w$  is the friction factor defined according to Soulsby (1995) and  $z_m$  is a reference depth above the sea bed (where the flow velocity is assumed to follow a logarithmic law). Simulations using two different definitions of  $z_m$  were carried out and their results were compared to provide a sensitivity analysis :

(H1).  $z_m$  is the depth of the grid cell point nearest the bottom,

(H2).  $z_m$  is a fraction of the mean depth.

Wave forcing terms of equations (1), (2), (3) and (4) are calculated using the mean wave parameters resulting from WW3. It solves the spectral wave action equation in space and time, from which spectrum based wave parameters, atmosphere-waves and ocean-waves parameters and many more parameters are derived. The main purpose of this model is to simulate the wave generation by wind, dissipation and redistribution effects, their propagation by solving :

$$\frac{DN}{Dt} = \frac{1}{\sigma} (\mathbf{S}_{ln} + \mathbf{S}_{in} + \mathbf{S}_{nl} + \mathbf{S}_{ds} + \mathbf{S}_{bot} + \mathbf{S}_{db} + \mathbf{S}_{tr} + \mathbf{S}_{sc} + \mathbf{S}_{ice} + \mathbf{S}_{ref} + \mathbf{S}_{mud}) \quad (7)$$

where  $\mathbf{N}(k, \theta; \mathbf{x}, t)$  is the wave action density spectrum which is a function of time ( $t$ ), physical space ( $\mathbf{x}$ ), wave number ( $k$ ) and wave direction ( $\theta$ ).  $\sigma$  is the intrinsic wave radian frequency. Source terms are  $\mathbf{S}_{ln}$ ,  $\mathbf{S}_{in}$ ,  $\mathbf{S}_{nl}$ ,  $\mathbf{S}_{ds}$ ,  $\mathbf{S}_{bot}$ ,  $\mathbf{S}_{db}$ ,  $\mathbf{S}_{tr}$ ,  $\mathbf{S}_{sc}$ ,  $\mathbf{S}_{ice}$ ,  $\mathbf{S}_{ref}$ ,  $\mathbf{S}_{mud}$ , respectively, for the linear wind input, exponential wind input, non-linear wind input, whitecapping dissipation, dissipation by bottom friction over sandy and rocky beds, depth-induced wave breaking dissipation, triad wave-wave interactions, bottom scattering, wave-ice interactions, reflection by shoreline or by floating icebergs and dissipation by viscous mud (more details can be found in Tolman and al. 2014).

For  $\mathbf{S}_{in} + \mathbf{S}_{ds}$ , formulations of Ardhuin et al. (2010) and Filipot and Ardhuin (2012) (hereinafter ST4), and Zieger et al. (2015) (hereinafter ST6) have been tested. They aim to

245 modelise the wind input, the swell dissipation and the wave breaking. Please note that ST4  
 246 and ST6 do not use a parametric tail in  $f^{-5}$  at high frequencies. For  $\mathbf{S}_{\text{bot}}$ , two parame-  
 247 terisations (hereinafter BT1 and BT4) from the JONSWAP (Hasselmann et al. 1973) and  
 248 SHOWEX (Ardhuin et al. 2003) experiments were evaluated. However, the results obtained  
 249 with Hasselmann et al. (1973) were not shown here. For  $\mathbf{S}_{\text{db}}$ , the expression of Battjes and  
 250 Janssen (1978) was chosen with the Miche-style shallow water limiter for maximum energy.  
 251 For  $\mathbf{S}_{\text{nl}}$ , the Discrete Interaction Approximation method (Hasselmann et al. 1985) was turned  
 252 on. For  $\mathbf{S}_{\text{ref}}$ , the parameterisation of Ardhuin and Roland (2012) was activated.

### 253 *c. Coupling Procedure and Numerical Set-up*

254 The two-way coupling procedure was initially built by Bennis et al. (2011, 2013). Now,  
 255 exchanges between the two models are managed by the automatic coupler OASIS (Valcke  
 256 et al. 2015), instead of PALM (Buis et al. 2008). We defined a coupling time step that  
 257 was greater than the models time steps. For each coupling time step, OASIS exchanges  
 258 hydrodynamic variables among the two models, which will serve to calculate the forcing  
 259 terms, as explained below. MARS computes hydrodynamic fields and sends, through the  
 260 OASIS coupler, the surface flow velocities, as recommended by Banihashemi et al. (2017),  
 261 and sea surface elevation to WW3. After several integration times, corresponding to one  
 262 coupling time step, WW3 sends mean wave parameters, e.g. significant wave height and  
 263 Bernouilli head, to MARS. The terms used in Eq. (1), (2), (3), (4), (6) are then calculated  
 264 by MARS from these mean wave parameters and the MARS hydrodynamic is re-computed.  
 265 Subsequently, the surface sea elevation and surface currents are re-sent to WW3 (see Figure  
 266 2), and so on.

267 We define two different coupling modes : i) the one-way mode (hereinafter OW) when  
 268 WW3 forces MARS and ii) the two-way mode (hereinafter TW) where the feedback from  
 269 MARS to WW3 is included in addition to the forcing of MARS by WW3.

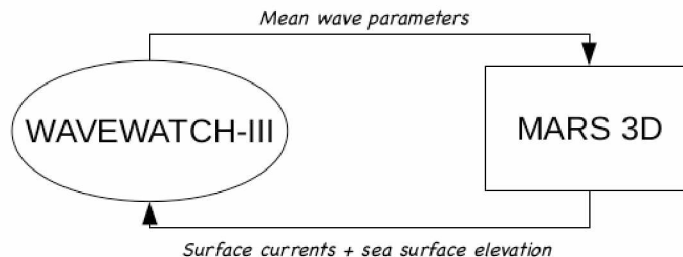


FIGURE 2: Coupling procedure. More details in Bennis et al. (2011).

271 Both models use two nested-grids (hereinafter parent and child grids), with similar  
 272 horizontal resolutions (600 m and 120 m), that are shown on Figure 1a. Their South-

West and North-East boundaries are : i) for parent grid : 47°53'60.0" N/6°03'32.4" W - 50°27'0.0" N/0°43'12.0" W, and ii) for child grid : 49°04'48.0" N/2°56'56.4" W - 50°4'12.0" N/1°23'24.0" W. All MARS simulations are in three dimensions with 12 sigma levels over the vertical. The wave model employs 32 frequencies from 0.04 Hz to 0.7678 Hz and 24 directions leading to a directional step of 15 degrees. Open boundaries of MARS are forced with the Shom CST France atlas that uses 114 tidal components (Leroy and Simon 2003). WW3 utilises wave spectra of the HOMERE and Ifremer databases (Boudiere et al. 2013) at its open boundaries. WW3 is forced by NCEP winds from CFSRR re-analysis. The deployment of child grids requires 2D-wave spectra, water levels and flow velocity from their parent grids at boundaries. All runs are coupled, with a one-way/two-way coupling for parent and child grids. The child grid coupling time step is 180 s and 20 s for one-way and two-way runs, respectively.

#### *d. Numerical Experiments*

A sensitivity analysis on the influence of main formulations and parameters is necessary to ensure a proper validation. The behaviour of the coupled model is assessed through different parameterisations for wave energy dissipation (ST4, ST6) and bottom friction (BT1, BT4). Moreover, the impact of bottom roughness ( $z_0$ ) and of the size of the near-bottom logarithmic layer ( $z_m$ ) are evaluated. Bottom stress in MARS is parameterised according to Eqs. (4), (5), (6) with (H1) and (H2) hypothesis for  $z_m$ . Tests are also carried out for the two coupling modes (OW and TW) in order to ensure cross validation. All sensitivity tests are not shown to avoid cluttering. So, only the relevant experiments were presented and they are summarised in Table 1. Runs 3, 4 and 7 included wave effects but not local wind effects while Runs 5 and 6 took into account the wave and local wind effects. The wave and wind effects were absent from Run 10 where the hydrodynamic was only driven by tides.

Model accuracy is evaluated through the root mean square error (RMSE), normalized root mean square error (NRMSE), BIAS, PBIAS, MAE and R-squared ( $R^2$ ), which are defined as follows (e.g. Allen et al. 2007b,a) :

$$\text{RMSE} = \frac{1}{N} \sqrt{\Sigma(X_{\text{model}} - X_{\text{data}})^2}, \quad (8)$$

$$\text{NRMSE} = \frac{\text{RMSE}}{\max(X_{\text{data}}) - \min(X_{\text{data}})}, \quad (9)$$

$$\text{BIAS} = \frac{\Sigma(X_{\text{model}} - X_{\text{data}})}{N}, \quad \text{PBIAS} = 100 \times \text{BIAS}, \quad (10)$$

$$\text{MAE} = \frac{\Sigma(|X_{model} - X_{data}|)}{N}, \quad (11)$$

$$R^2 = 1 - \frac{\Sigma(X_{data} - X_{model})^2}{\Sigma(X_{data})^2}, \quad (12)$$

where  $N$  is the total number of available samples,  $X_{model}$  and  $X_{data}$  are related to samples coming from numerical simulations and in-situ data, respectively. PBIAS gives a measure of whether the model is systematically underestimating or overestimating the measurements. The closer the value is to zero the better the model. Performance levels regarding |PBIAS| are categorised as follows  $\leq 10$  excellent,  $10 - 20$  very good,  $20 - 40$  good,  $\geq 40$  poor (Marechal 2004; Allen et al. 2007a).  $R^2$  is a statistical measure of how close the data to the fitted regression line.  $R^2 = 1$  indicates that model results and data are similar. Performance levels regarding  $R^2$  are categorised as :  $\geq 0.65$  excellent,  $0.65 - 0.5$  very good,  $0.5 - 0.2$  good,  $\leq 0.2$  poor (Marechal 2004). The choice of category boundary is subjective, these criteria are not of the fail/pass type, but valuate the performance in four categories from excellent to poor.

	Year	Wave energy dissipa- tion	Wave bottom friction	Coupling mode	Local wind ef- fects
<b>Run 3</b>	2017	ST6	BT4	TW	NO
<b>Run 4</b>	2017	ST6	BT4	OW	NO
<b>Run 5</b>	2017	ST4	BT4	OW	YES
<b>Run 6</b>	2017	ST4	BT4	TW	YES
<b>Run 7</b>	2017	ST4	BT4	TW	NO
<b><i>Run 10</i></b>	<i>2017</i>	—	—	—	NO

TABLE 1: List of numerical experiments according to date, wave energy dissipation formulation, wave bottom friction parameterisation, coupling mode and the inclusion of local wind effects. All runs include wave effects except for Run 10 in italics.

### 3. Results and Discussion

Model tests against ADCP, wave buoys and tidal gauge data are presented. Numerical validations are related to tidal elevation, mean wave parameters, wave spectra, time series and

vertical profiles of the tidal stream velocity. We investigated how ocean waves interact with the tidal current in Alderney Race for different met-oceanic conditions. Effects of bottom friction, bottom roughness, direction of propagation of wave and current, and turbulence modelling are discussed.

#### *a. Tidal Elevation*

Tidal range varies between 4 and 7 m for the studied area and time period. Comparisons between measurements of tidal gauges (TG1 and TG2) and numerical simulations of MARS-WW3 are shown on Figure 3. Our coupled model produces mean sea surface elevation values with a terrestrial definition (IGN 69) for the vertical reference. As TG1 and TG2 measurements use the levels of the lowest tide, chart data as vertical references, we shifted the simulated water level with 3.88 m for TG1 and 5.55 m for TG2 as recommended by Shom (2017) to provide a commensurable comparison. This correction, based on the minimum BIAS, is consistent with the measured mean sea level of 3.87 m and 5.45 m in Cherbourg and Dièlette in 2017, respectively (Shom 2017).

Model and data results were close at Cherbourg (TG1), with good fits in amplitude and time phasing (see Figure 3, black dots and red line). Absolute error (hereinafter AE) were around few tens of centimetres, with a maximum values of 0.39 m (see Table 2). RMSE was 0.13 m and  $R^2 = 0.98$ , that is excellent. Errors mainly occurred just before the high tide, showing that the tidal asymmetry was not well represented in some cases. At Dielette (TG2), numerical simulations were worse than in Cherbourg but they were acceptable, with  $BIAS = 0.02$  m,  $RMSE = 0.44$  m and  $R^2 = 0.95$ . However, discrepancies were observed with a phase delay up to few minutes for some tidal cycles. In contrast, this problem was absent in Cherbourg (TG1). This illustrates the complexity of the tidal dynamic around La Hague Cape where the tidal range increases by 5 metres within a few kilometres, as shown in Bailly Du Bois et al. (2012). This could be ascribed to bathymetry errors and bottom stress that is strongly impacted by such errors (more details in Section 3c).

		<b>21-25 Nov 2017</b>
<b>TG1 (Cherbourg)</b>	max(AE)	0.39 m
	min(AE)	-0.16 m
	BIAS	0.06 m
	RMSE	0.13 m
<b>TG2 (Dièlette)</b>	max(AE)	0.73 m
	min(AE)	-0.34 m
	BIAS	0.02 m
	RMSE	0.44 m

TABLE 2: Maximum (max(AE)) and minimum (min(AE)) values of AE are presented as well as BIAS and RMSE for TG1 and TG2. Positive and negative signs denote under-estimation and over-estimation of water levels by the model, respectively.

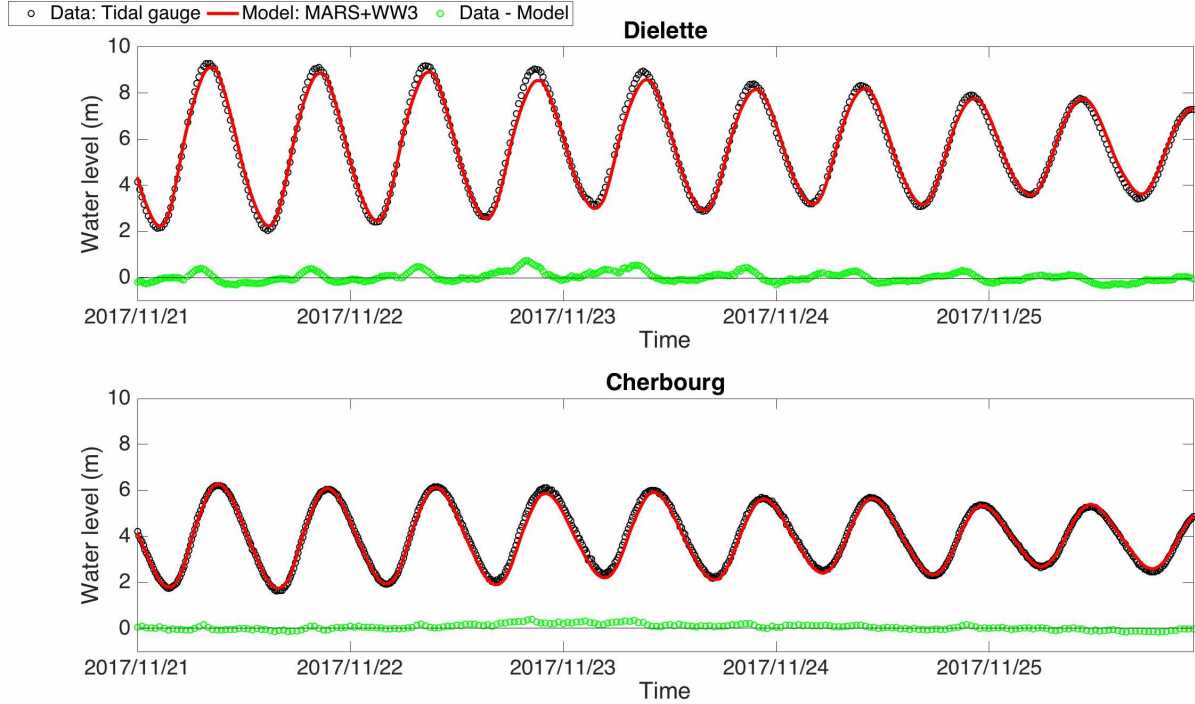


FIGURE 3: Water level at Dielette (TG2, top row) and Cherbourg (TG1, bottom row) measured by Shom tidal gauge (black dots) and computed by the coupled model (red solid line) over 5 days from 21 to 25 November 2017. AE is represented in green dots at each time.

#### b. Sea States

Sea states in Alderney Race are often complex, with wind seas combined with swells from the Atlantic Ocean (Maisondieu 2016). Comparisons between numerical simulations and

measurements were performed in order to investigate how wave-current interactions influence ocean waves in Alderney Race. Several parameterisations for the wave energy dissipation by whitecapping and bottom friction were evaluated. Moreover, water level and surface current effects on the wave field are presented and discussed as well as local wind effects.

Time series of the magnitude (hereinafter  $U_{10}$ ) and direction (hereinafter  $Udir_{10}$ ) of the wind at 10 metres above ground level (see Figure 4) showed high values for  $U_{10}$  during the night of 22-23 November, with a maximum value around 17.5 m/s, and for a North-North-East to South-South-West wind. The wind simulated by CFSRR, that have a spatial resolution of  $0.2^\circ$  of latitude and of  $0.1^\circ$  of longitude at the study site location, is used to force WW3. Wind forcing was in agreement with the wind measured at Goury by the semaphore (see Figure 4). NRMSE is around 0.11 (see Table 3) while PBIAS is positive for  $U_{10}$ , indicating that the CFSRR values were higher than the measured ones. However, PBIAS remains very good for  $U_{10}$  (around 14%) while MAE is excellent for  $Udir_{10}$  (around 7%). The discrepandancies can be explained by the coarse resolution of the CFSRR model, the distance (around 7 km) between Goury and the ADCP point (Coelingh et al. 1996, 1998), and also because the semaphore data are recorded above ground level that influences the atmospheric boundary layer and the wind velocity (e.g. Bailly Du Bois and Dumas 2005). The significant wave height recorded by ADCP was highest on 23 November at 2 :13 a.m. and 3 :13 a.m., reaching 4 m and 3.6 m, respectively. During this time period, high winds were measured with  $U_{10}$  values greater than 15 m/s. As a result the inclusion of local wind effects has improved the simulated significant wave height (see Figures 5 and 6a, Run 3 vs Run 6, and Table 4), in particular between the 22 November at 12 a.m. and the 23 November at 12 p.m, where  $U_{10}$  was highest. NRSME has been reduced by 50% and now reached 0.08 for Run 6. PBIAS were high for Run 3 (around 25%) and Run 4 (around 28%) and showed that the significant wave height was largely underestimated by the model. With local wind effects, PBIAS decreased substantially to 5.8% for Run 6 (see Table 4), that is excellent. R-squared values and scatter plots of Figure 5 well illustrated how the local wind effects have improved the fit to data, with  $R^2 = 0.97$  for Run 6 instead of 0.87 for Run 3.

The wave-to-ocean momentum flux is enhanced due to local wind effects, particularly for the zonal component, which was 60-fold increase, when wind blows hard (on 23 November around 2 a.m). This increase is ascribed to changes in both wind speed (from 12 m/s on 22 November around 12 p.m. to 17.5 m/s on 23 November around 2 a.m) and direction (from South-South-West direction on 22 November around 12 p.m. to West direction on 23 November around 2 a.m) during the storm. Wave direction was worse for simulations with local wind effects between the 22 November at 12 a.m. and the 23 November at 12 p.m. compared to the simulations without such effects (see Figure 6b, Run 3 vs Run 6). With local wind effects, waves tend to go towards the North everytime instead of turning East



(more explanations hereafter). From the 23 November at 12 p.m to 25 November 11 p.m., wave direction fitted well to the observations. Therefore, only this time period will study in the next section (3c), which deals with waves effects on tidal currents, because changes in currents due to the waves are partly driven by the direction of propagation of waves, that should be well represented to perform a right analysis.

One of most important physical phenomenon in Alderney Race is the wave refraction ascribed to the strong tidal current. This phenomenon was well simulated by the coupled model, particularly when the local wind effects were not included in simulations (see Figure 6b, Runs 3 and 6) : refraction has modified wave direction, which was in the agreement with observations and former studies (e.g. Wolf and Prandle 1999; Ardhuin et al. 2012). When wind blowed hard, local wind effects (see Figure 6b, Run 6) tended to smooth refraction effects because currents were abnormally reduced (more details in section 3c) and therefore they had less influence on surface waves. Runs 4 and 5, which did not include neither current effects on waves nor local wind effects, failed to correctly reproduce the measured wave direction. A modulation of significant wave height was also observed due to refraction (Figure 6a; Runs 3 vs 4 or Run 5 vs 6). Both parameterisations for wave breaking dissipation (ST4 and ST6) adequately simulated modifications in the significant wave height and wave-to-ocean momentum flux by tide (see Figures 6a,c,d; Runs 3 vs 4 or Run 5 vs 6). The eastward and northward components of the momentum flux displayed peak values during the ebb, when the tidal current was southwestward (see Figures 6c,d; Runs 3 vs 6). In that case, interactions between ocean waves and tide generated wave breaking events that produced an enhanced wave-to-ocean momentum flux. This is highly visible if we compare the results of the two coupling modes (see Figure 6c,d; Runs 3 vs 4 or Run 5 vs 6) : peaks were absent from Runs 4 and 5 because they did not take into account the current effects.

Tide also influences the near-bed orbital velocity, and particularly its meridional component (Figure 6f, Run 3 and 6) because of the tidal current direction, that was NNE/SSW. For the Runs 3 and 6, near-bed orbital velocity was modulated by tides with high and low values during ebb and flood, respectively. In contrast, Runs 4 and 5, being computed without interactions with the flow, did not have such peaks (see Figure 6f), showing the impact of wave-current interactions. The zonal component of the near-orbital velocity was the highest due to the direction of wave propagation, that was mainly from West to East. Its form resembles significant wave height, with maximum values during the night of 22-23 November (see Figure 6e). The effects of local wind on the near-bed wave orbital velocity are light in comparison with the tidal ones, except for the 23 November around 2 a.m. where an increase of 5 cm/s was observed due to the strong wind. On the whole, parameterisations for wave bottom friction of Hasselmann et al. (1973) and Ardhuin et al. (2003) produced close near-bed results.

The wave-current interactions are also visible on the frequency wave energy spectra (see Figure 8). To study it in details, we selected eight moments that differ in terms of type of wave-current interactions (various waves and current directions, low/high tide, flood/ebb tide, low/high flow velocity). All related informations are summarised in Table 5. All presented spectra were bi-modal with a swell component, where the maximum of energy was located in  $f_{p,swell} = 0.07812 \text{ Hz}$  (RTF2, RTE1 and RTE2) and  $f_{p,swell} = 0.09375 \text{ Hz}$  (RTF1), and a wind-wave component, which reached its maximum in  $f_{p,windsea} = 0.125 \text{ Hz}$  (RTF2, RTE1 and RTE2) and  $f_{p,windsea} = 0.1406 \text{ Hz}$  (RTF1). The splitting frequency ( $f_c$ ) is around  $0.11 \text{ Hz}$ . For the swell component, all runs produced similar results, that are in agreement with the ADCP measurements regardless of coupling mode, wave dissipation parameterisation and local wind effects (see Figure 8). For the wind component, a wave energy decay in  $f^{-4}$  for frequencies between  $f_{p,windsea}$  and  $3f_{p,windsea}$  is observed, as demonstrated by Toba (1973), Donelan et al. (1985) and others. When frequencies were greater than  $3f_{p,windsea}$ , a decay in  $f^{-5}$  is found, as defined in Phillips (1958). Numerical results were consistent with the ADCP data for all runs up to  $2f_{p,windsea}$ . Runs 5 and 6, which have integrated local wind effects in simulations, overestimated the wave energy beyond  $2f_{p,windsea}$  and had an energy tail in  $f^{-4}$ . Run 7, which used the same parameterisation for the wave energy dissipation (ST4) than Runs 5 and 6, did not suffer to this overestimation, showing that the influence of local wind effects on ST4.

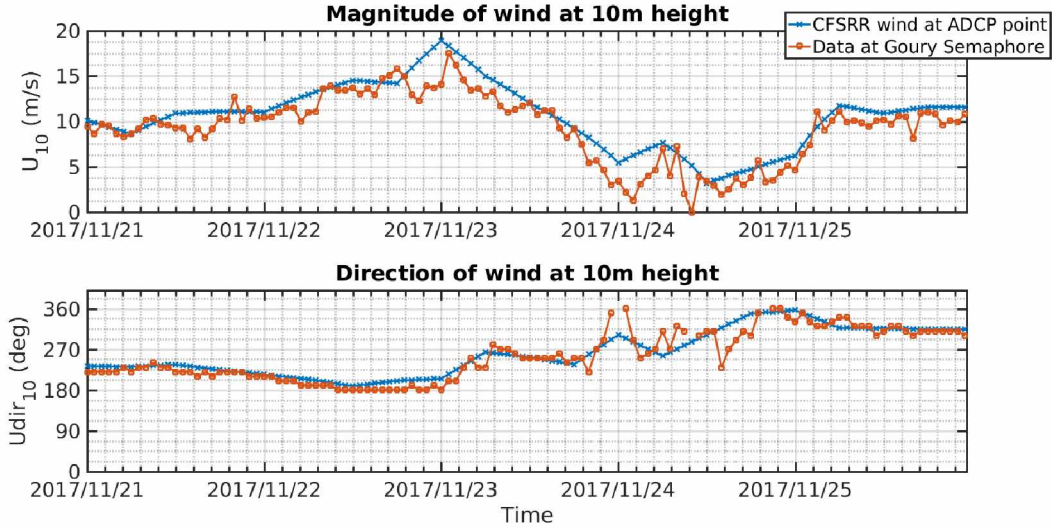


FIGURE 4: Time series for  $U_{10}$  (top panel) and  $Udir_{10}$  (bottom panel) : CFSRR inputs are in blue and data from the Goury semaphore are in red.

	NRMSE	$R^2$	PBIAS (%)
$U_{10}$	0.10	0.96	13.97
$Udir_{10}$	0.11	0.98	3.82

TABLE 3: NRMSE,  $R^2$  correlation and PBIAS for  $U_{10}$  and  $Udir_{10}$ .

	<b>Runs</b>	<b>NRMSE</b>	<b>R<sup>2</sup></b>	<b>PBIAS (%)</b>
<b>H<sub>s</sub></b>	Run 3	0.16	0.87	-27.95
	Run 4	0.16	0.87	-25.57
	Run 5	0.10	0.95	12.07
	Run 6	0.08	0.97	5.89

TABLE 4: NRMSE, R<sup>2</sup> correlation and PBIAS for significant wave height (H<sub>s</sub>). Runs 3, 4, 5 and 6 are presented.

	<b>Date</b>	<b>W ⊥ C</b>	<b>W+C</b>	<b>U<sub>c</sub><sup>s</sup>(m/s)</b>	<b>SSH(m)</b>
<b>RTF1</b> <b>(flood)</b>	23/11/2017 10 :45 p.m.	X		2.01	1.87
<b>RTF2</b> <b>(flood)</b>	24/11/2017 00 :45 a.m.	X		1.13	0.67
<b>RTE1</b> <b>(ebb)</b>	24/11/2017 02 :45 a.m.		X	0.93	-1.18
<b>RTE2</b> <b>(ebb)</b>	24/11/2017 03 :45 a.m.		X	1.99	-1.82
<b>RTF3</b> <b>(flood)</b>	25/11/2017 11 :00 a.m.	X		1.87	1.76
<b>RTF4</b> <b>(flood)</b>	25/11/2017 01 :45 p.m.		X	0.63	0.11
<b>RTE3</b> <b>(ebb)</b>	25/11/2017 04 :15 p.m.		X	1.56	-1.47
<b>RTE4</b> <b>(ebb)</b>	25/11/2017 05 :00 p.m.		X	2.07	-1.73

TABLE 5: Waves-current direction (W ⊥ C when waves and current direction are orthogonal and W + C for an angle between waves and current direction less than 80°), surface current velocity (U<sub>c</sub><sup>s</sup>) and sea surface height (SSH) at RTF1, RTF2, RTE1, RTE2, RTF3, RTF4, RTE3 and RTE4. Directions follow the oceanographical convention for the flow and the meteorological convention for waves. Values are from numerical simulations.

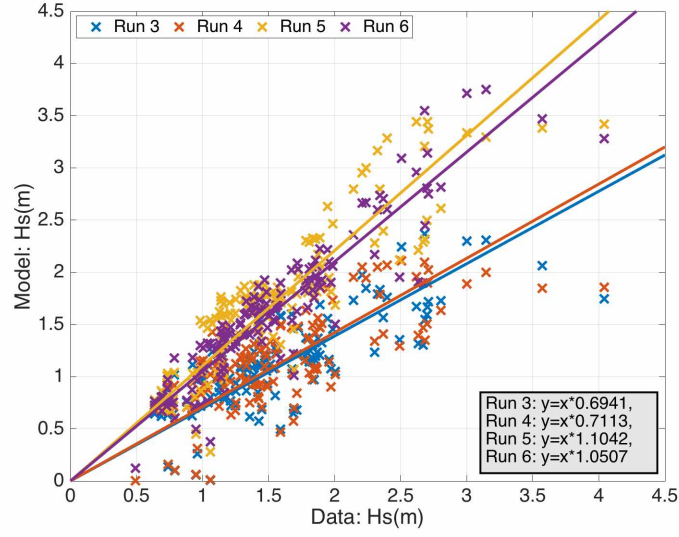


FIGURE 5: Scatter plots for the significant wave height ( $H_s$ ) for Run 3 (blue crosses), Run 4 (red crosses), Run 5 (yellow crosses) and Run 6 (purple crosses). In-situ data and model results are drawn along x-axis and y-axis, respectively. Regression lines are plotted in blue, red, yellow and purple solid lines for runs 3, 4, 5 and 6, respectively.

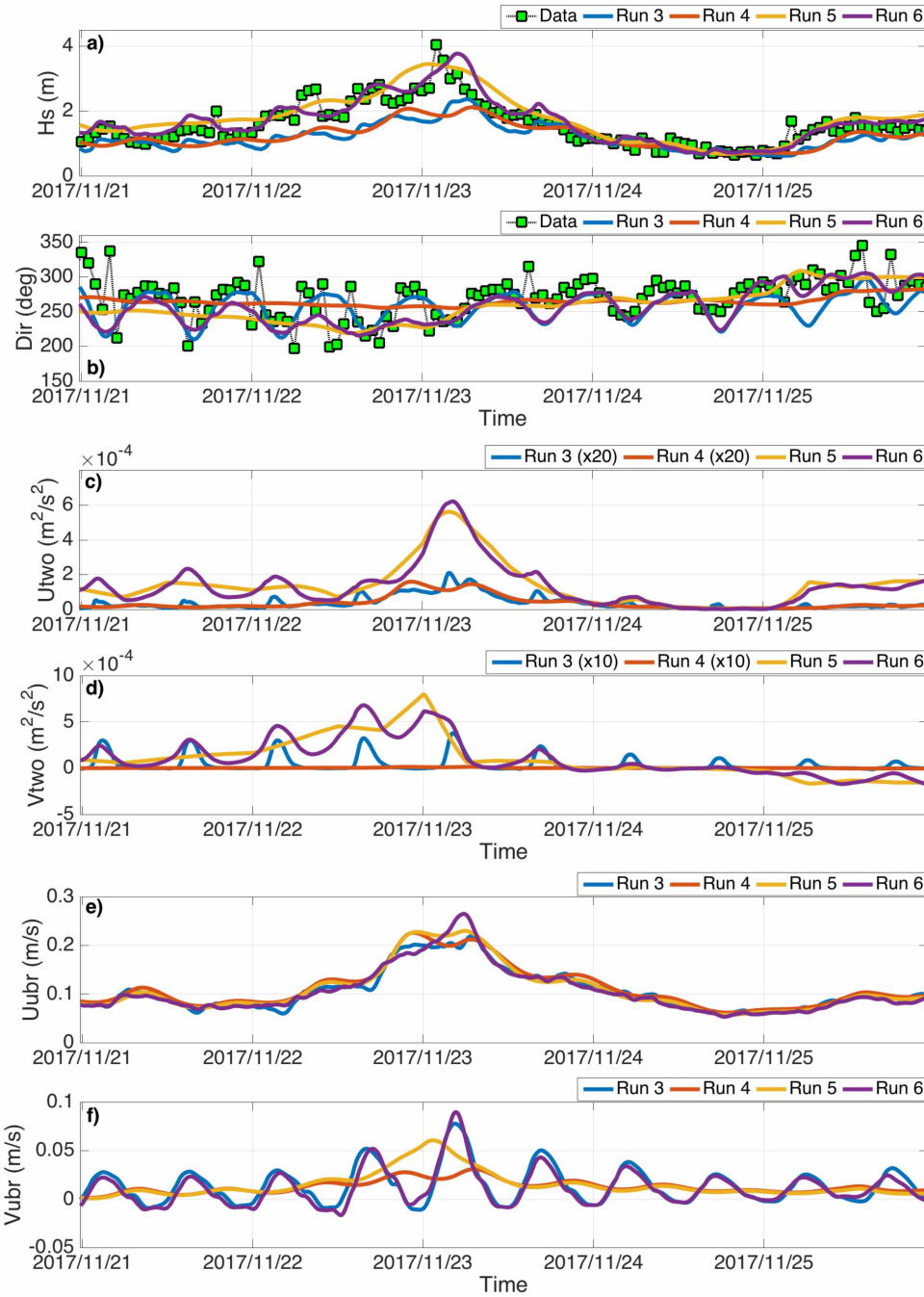


FIGURE 6: Time series of sea states characteristics, from 21 to 25 November 2017, integrated over frequencies (32) and directions (24) : a) significant wave height, b) wave direction (meteorological convention), c) zonal component of wave-to-ocean momentum flux, d) meridional component of wave-to-ocean momentum flux, e) zonal component of near-bed wave orbital velocity, f) meridional component of near-bed wave orbital velocity. ADCP data are in black-green squares while numerical results for Runs 3, 4, 5 and 6 are in blue, red, yellow and purple solid lines, respectively.

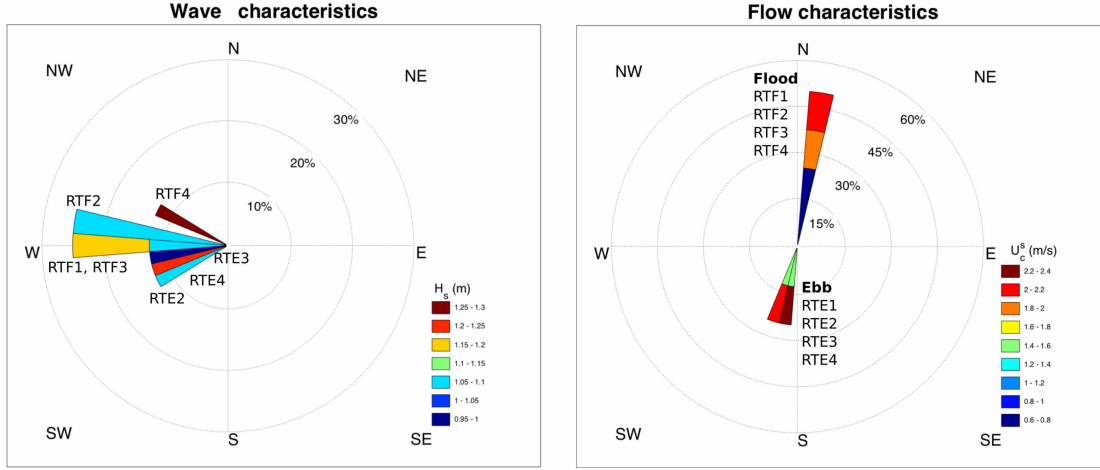


FIGURE 7: Left panel : Wave direction (polygon) and significant wave height (color). Right panel : Direction (polygon) and velocity magnitude (color) of the current. Only RTF1, RTF2, RTE1, RTE2, RTF3, RTF4, RTE3 and RTE4 are shown. Directions follow the oceanographical convention for the flow and the meteorological one for waves. Results are from numerical simulations.

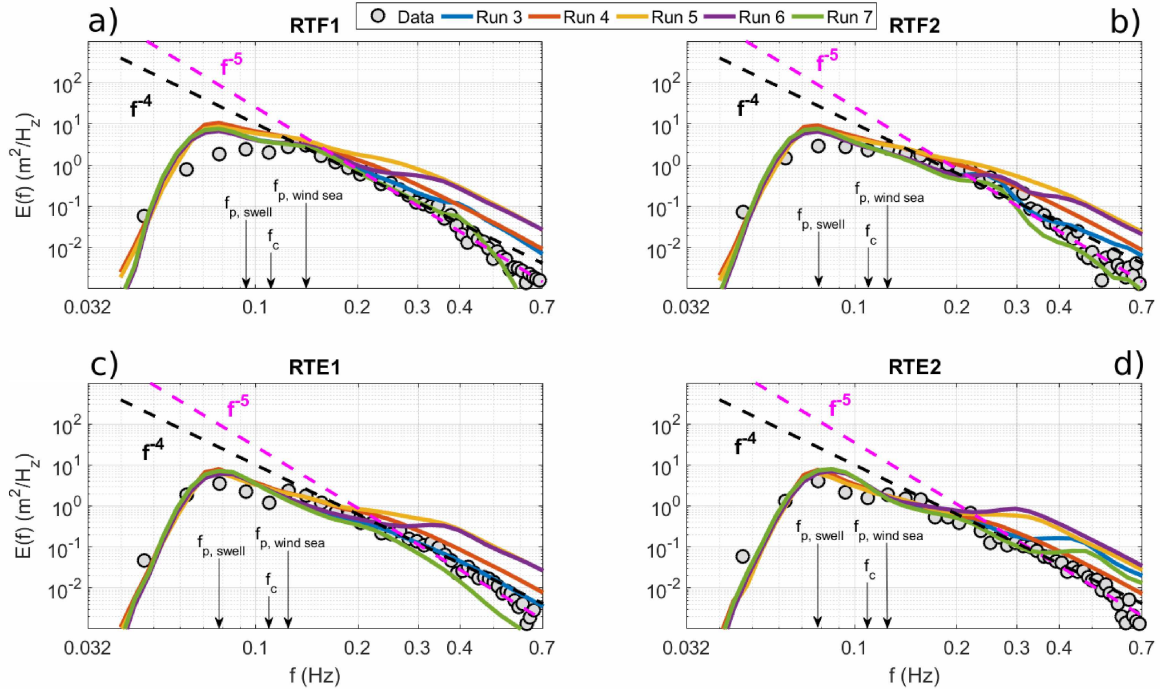


FIGURE 8: Frequency wave energy spectra at : a) RTF1, b) RTF2, c) RTE1 and d) RTE2. The first (a,b) and second (c,d) rows are for flood and ebb tides, respectively. Magenta and black dashed lines are for  $o(f^{-5})$  and  $o(f^{-4})$ , respectively. Note that the x-axis and y-axis are a log10 scale.  $f_{p,swell}$ ,  $f_{p,windsea}$ ,  $f_c$  (around  $0.11 H_z$ ) represent the peak frequency for swell and wind sea, and the splitting frequency, respectively.

### *c. Time Series of the Tidal Stream Velocity*

We now investigate how the Alderney Race circulation was impacted by ocean waves. Figure 9 shows time series of magnitude and direction of the measured current velocity at 25, 12 and 6 meters depth above seabed at the ADCP location between 23 November 2017 at 12 p.m and 25 November 2017 at 8 p.m, where the tidal flow velocity varied between 0 and 2.5 m/s. First, we start by discussing the results without local wind effects (Runs 3 and 10). On the whole, the numerical results for cases with (Run 3) and without waves (Run 10) were consistent with the data and were close each other (see Figures 9 and 10 and Table 6). Good and similar NRSME (0.09 and 0.11) and  $R^2$  (0.97 and 0.98) for both runs were observed. PBIAS were excellent, because they range from 0.6% to 3.8%, but they showed that velocity was underestimated by the model when wave effects were activated while an overestimate occurred where wave effects were disabled. Wave effects tend to reduce systematically the velocity magnitude (e.g. Grant and Madsen 1979; Xie et al. 2001; Zhang et al. 2004) due to wave enhancement of bottom friction. At low tide, as the model without waves overestimated tidal velocity, simulations with waves had a better fit with in-situ data. Flow direction (see Figure 9d), and its changes belonging to the tidal cycle were well reproduced by the model. Close results were obtained for Run 3 (TW mode) and Run 4 (OW mode, not displayed here). Even if current and sea level produced significant changes in the wave field (see Figure 6), the impact of feedback on the tidal current and water level remained weak.

As regards the cases taking into account local wind effects (Runs 5 and 6) and between the 23 November 12 p.m. and 25 November 2 a.m., tidal current were little impacted by local wind effects and numerical results were close to the data (see Figure 9). Indeed, NRMSE (around 0.11),  $R^2$  (around 0.97) and PBIAS (0.62% – 3.79%) were in the same order of magnitude for Run 3 (without local wind) and Run 6 (with local wind). That could be explained by the wind speed at the ADCP point which was weak for this time period, ranging from 4 to 10 m/s (see Figure 4). Beyond the 25 November at 2 a.m., when the wind started blowing, a strange behaviour was observed, particularly during the flood with a change in current direction, that induced a loss in the current intensity of around 0.5 m/s, with a smoother transition between ebb and flood directions and a shift in direction to the East. As the tidal current direction was modified, wave-current interactions were impacted. The decrease in velocity magnitude being similar for Runs 5 and 6, that is not induced by a change in wave field due to wave-current interactions. During this time period, waves went towards the East (see Figure 6b), and therefore the change in tidal current direction has reduced the angle between waves and current, leading to a decrease in the current intensity, as reported by Groeneweg and Klopman (1998). As a result, this problem comes from the ocean model MARS and suggests a mis-evaluation of the wind effects on the flow and particularly of the wind stress. Further investigations are required.

We noted an occasional phase delay that varied over time from 0 to 30 min. The delays differed depending to the tidal cycles. For example, in tidal cycles containing RTF1, RTF2, RTE1 and RTE2, the phase delay was constant at 15 min and in the same direction for all runs, regardless the coupling mode and the local wind effects. In contrast, we noted different time delays at RTF3 (no time delay for runs with and without local wind effects), RTF4 (30 min and no time delay for runs without and with local wind effects, respectively), RTE3 (30 min and no time delay for runs without and with local wind effects, respectively) and RTE4 (15 min for runs with and without local wind effects) times, which were part of the same tidal cycle. It is interesting to see that the delay was removed in simulations with local wind effects at RTF3, RTF4 and RTE3. That suggests that time delay could be due to waves and wind through the bottom stress, which was based on the near-bed wave orbital velocity. In addition, the maximum value of 30 min often corresponded to the reverse tide occurring sooner in the in-situ dataset. At this time, waves effects on the current were strongest. To investigate the role of waves and wind, measurements of the near-bed wave orbital velocity are required.

Furthermore, time delay could be partially corrected by expressing  $z_m$  (Eq. (5)) as a fraction of the mean depth ( $D$ ). In this study and after many simulations, the fraction was set to 5%. Some sensitivity tests were performed with  $z_m$  at 50% (Uchiyama et al. 2009), 26.7%, 11.4%, 5.71%, 2.67% and 1.33% of  $D$  and the related velocities are presented in Figure 11. A one-hour delay was observed, as shown on Figure 11, between both extreme cases (50% and 1.33% of  $D$ ), highlighting the link between the bottom stress formulation and time delay. Tidal asymmetry also appeared also to be sensitive to the  $z_m$  value, with differences between tidal phases that were accentuated by large  $z_m$  values. Because the depth where the logarithmic profile is imposed was strongly dependent on the turbulence level, these tests showed that one of the sources of the time delay was ascribed to the modification of the turbulence level by the interactions between seabed morphology, ocean waves and tidal current. Parameterisation using an apparent roughness as recommended by Grant and Madsen (1979); Signell et al. (1990); Mathisen and Madsen (1999) may be a way of improvement, in addition to use of high spatial resolution bathymetry based on Furgerot et al. (2019).



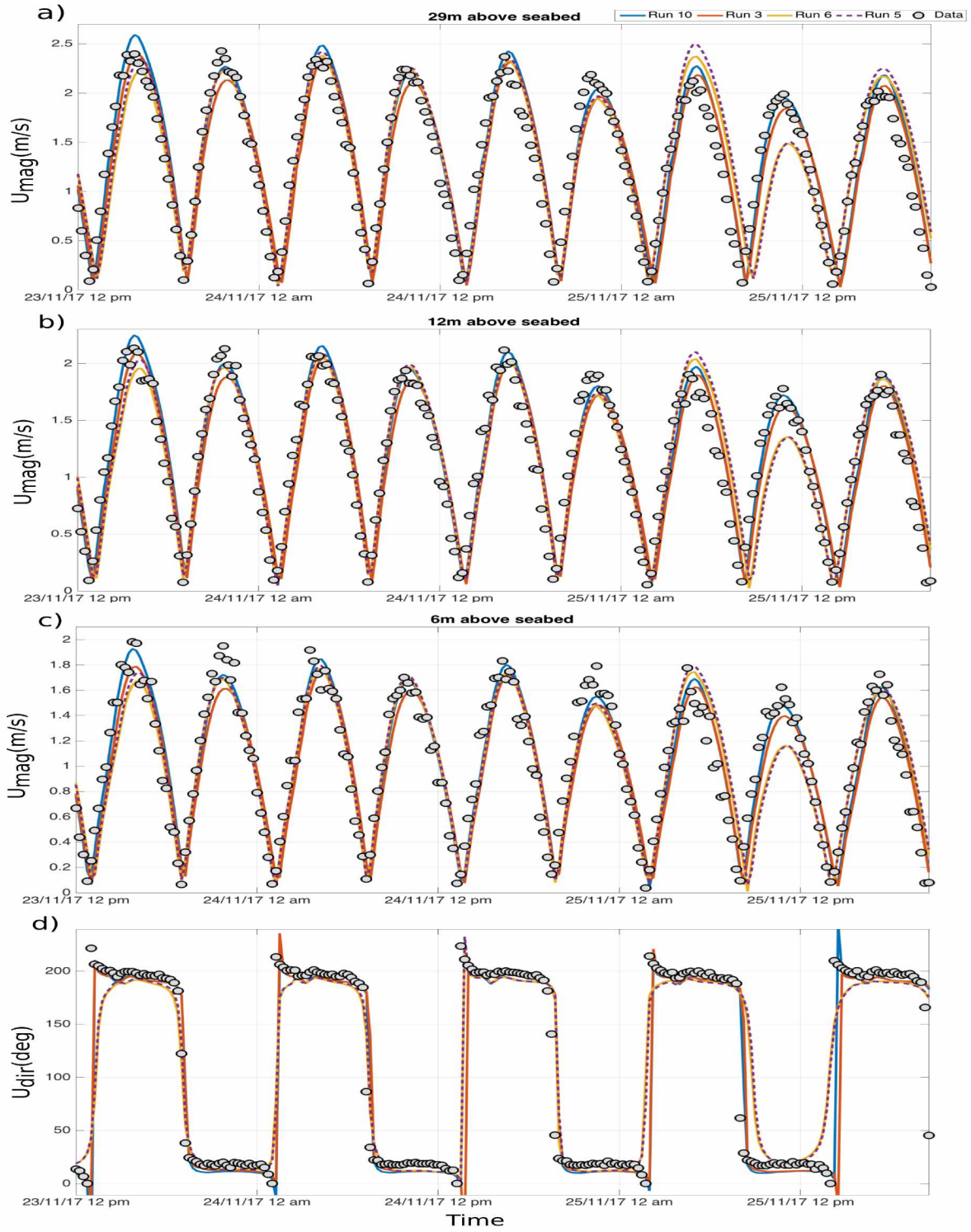


FIGURE 9: Time series of magnitude (a,b,c) and direction (d) of the current velocity at 29 m (a), 12 m (b,d), 6 m (c) depth above seabed. All panels show numerical results for Runs 3 (TW, red solid line), 5 (OW+local wind effects, purple dash line), 6 (TW+local wind effects, yellow solid line) and 10 (without waves, blue solid line). In-situ data are shown with black circles. Directions are based on oceanographic conventions.

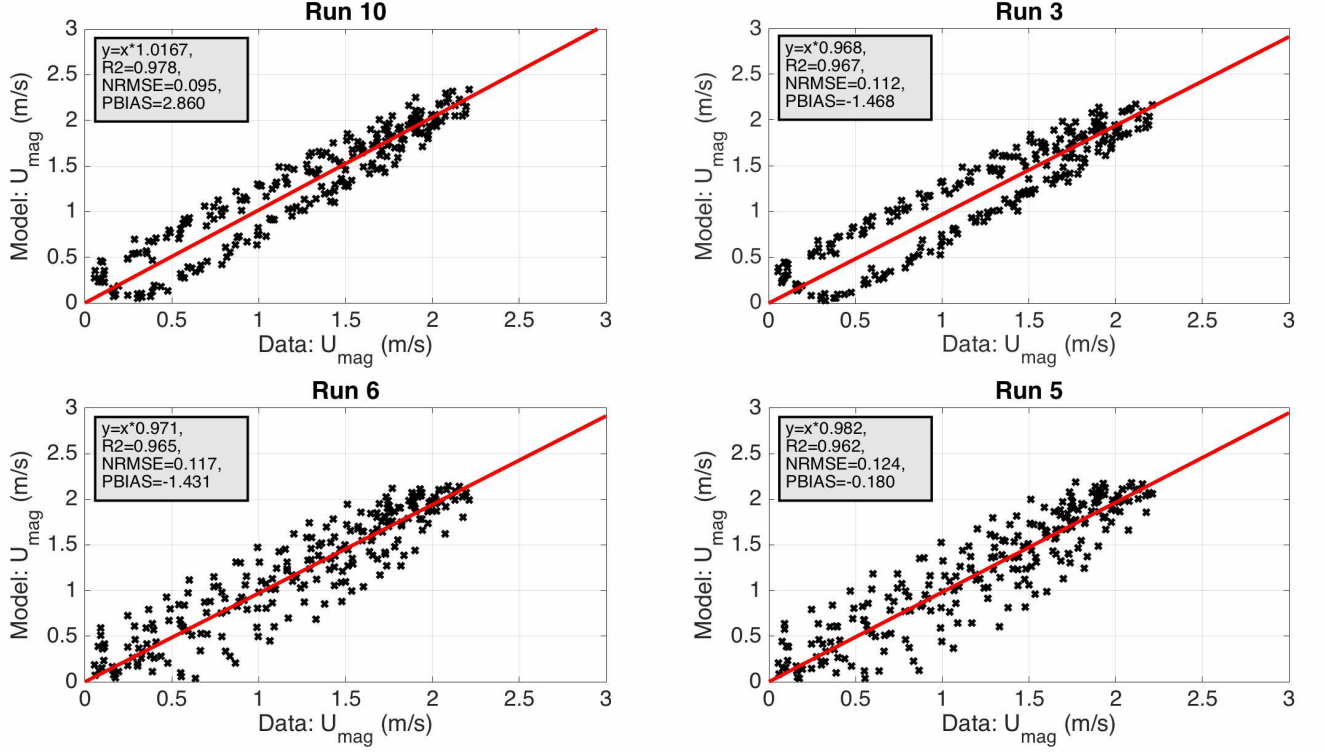


FIGURE 10: Scatter plots for the velocity magnitude (black crosses) at 15 metres above seabed for Run 10, Run 3, Run 6 and Run 5. In-situ data and model results are drawn along x-axis and y-axis, respectively.

	Runs	NRMSE	$R^2$	PBIAS (%)
<b>z = 29 m</b>	Run 3	0.11	0.97	-0.62
	Run 5	0.14	0.95	1.92
	Run 6	0.12	0.96	-0.41
	Run 10	0.10	0.98	3.76
<b>z = 12 m</b>	Run 3	0.11	0.97	-1.78
	Run 5	0.12	0.96	-0.44
	Run 6	0.11	0.97	-1.66
	Run 10	0.09	0.98	2.58
<b>z = 6 m</b>	Run 3	0.10	0.97	-3.79
	Run 5	0.11	0.96	-2.33
	Run 6	0.11	0.96	-3.53
	Run 10	0.09	0.98	0.74

TABLE 6: NRMSE,  $R^2$  correlation and PBIAS for velocity magnitude at different depths (29 m, 12 m and 6 m). Runs 3, 5, 6 and 10 are presented.

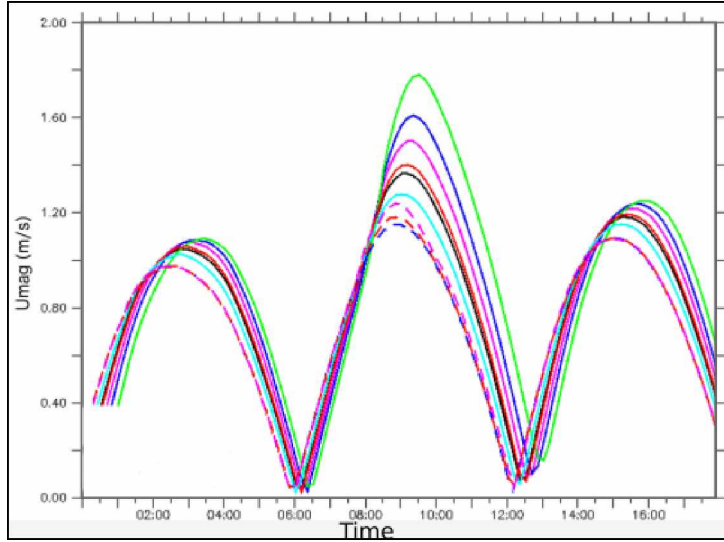


FIGURE 11: Time series of the velocity magnitude for different values and formulations of  $z_m$  parameter. Simulations do not include wave effects and used a  $z_0$  equal to  $0.008m$ . Reference test was based on the quadratic formulation (black solid line). Green line is for Uchiyama et al. (2009) using  $z_m = 50\%D$ . Other lines represent current velocity simulated with  $z_m$  set to  $26.7\%D$ ,  $11.4\%D$ ,  $5.71\%D$  and  $2.67\%D$  for the solid blue, magenta, red, cyan lines. Dashed blue, red and magenta lines are for  $z_m = 1.33\%D$  and using Okubo (1971) with  $f_{visc} = 17$ ,  $f_{visc} = 10$  and Smagorinsky (1963), respectively.

#### d. Vertical Structure of the Tidal Stream Velocity

The vertical shape of the tidal stream velocity in Alderney Race is known to depend primarily on the tidal phase, surface wave effects, bottom friction and turbulent mixing. Thibault and Sentchev (2017) and others (e.g. Lewis et al., 2017) found that, in some particular cases, the vertical shear of the tidal sea current follows a power law (see Eq. (1) of Lewis et al., 2017) scaled with a roughness coefficient. This form fits well with their data for a calm sea. Otherwise, they showed that ocean waves, when their effects are significant, change the shear flow and increase the error between model and measurements. They recommend the use of a 3D fully wave-current model, which is what we used here. It is not easy to compare vertical profiles of the flow to observations in a location such as Alderney Race. Indeed, there are some difficult issues : i) the very rough bottom ejects intermittent 3D highly energetic turbulent eddies that modify the vertical shape of the flow, ii) the phase delay complicates the comparison : all presented plots are corrected by phase delay by adjusting the tide reversal time, iii) measurements near the surface lack precision because they have been filtered to eliminate spurious values induced by the acoustic signal reflection on the sea surface, but

also by the air bubbles from wave breaking turbulence, and iv) ocean waves and current are orthogonal most of the time, which brings us further away from simple case where ocean waves and current directions are aligned or opposite. We need to define a scale to perform the analysis : waves and current are considered as opposite for  $|\Delta\theta_{w/c}| = 180^\circ$ , aligned for  $|\Delta\theta_{w/c}| = 0^\circ$  and orthogonal for  $|\Delta\theta_{w/c}| = 90^\circ$ , with  $|\Delta\theta_{w/c}|$  the angle between waves and tidal current directions of propagation. Despite these issues, we compared vertical profiles to in-situ measurements.

Comparisons between model and data results were carried out at RTF1, RTF2, RTE1, RTE2, RTF3, RTF4, RTE3 and RTE4, where tidal current and wave directions differed as well as tide conditions (see Table 5). Some statistical calculations were performed for the velocity magnitude. NRSME varied from 0.02 to 0.15, except at RTF3 where a value of 0.36 was reached in case of Run 6. R-squared ranged from 0.96 to 0.99 except at RTF3 for Run 6 where it was around 0.92. PBIAS showed that sometimes model overestimated the measured velocity and sometimes underestimations occurred. PBIAS were less than 4%, which is excellent, except for Run 6 at RTF2 and RTF3 where they were around 21%. Therefore, statistical parameters showed a very good agreement ( $R^2 \geq 0.96$ ,  $\text{NRMSE} \leq 0.15$ ,  $\text{PBIAS} \leq 4\%$ ) between in-situ measurements and model results for all runs, except for Run 6 at RTF3 and RTF2. Discrepancies for Run 6 are due to the mis-evaluation of local wind effects in the ocean model when wind blew hard, as explained before. However, these parameters do not allow us to analyse if the vertical shapes were along the right direction.

For flood cases, at RTF1, RTF2, RTF3, wave and current directions were orthogonal while at RTF4  $|\Delta\theta_{w/c}|$  was around  $80^\circ$ . At RTF4, simulations with wave effects (Runs 3 and 6) produced higher velocity than the one without wave effects (Run 10). The wave forcing had improved the results, that were consistent with the ADCP data. Runs 6 and 3 velocities were different by their vertical structure : from 20 to 30m depth above seabed, Run 6 velocity was reduced while, for Run 3, the velocity was increased. Both vertical structures being in agreement with the measurements, it is difficult to conclude. However, these forms in the upper part of the water column represent different type of wave-current interactions : aligned waves and current cause a decrease in surface flow while opposite waves and current accelerate the surface flow due to Stokes drift effects (e.g. Groeneweg and Klopman 1998). As explained previously, at RTF4, when local wind effects were included in the simulations (Run 6), the angle between waves and current tended to become small, and as a result the surface flow velocity was decreased. That what we are seen in Figure 12.

At RTF2, inappropriate boosting of the flow was visible when the wave forcing is activated, while Run 10 (without waves) fitted well to the data. The inclusion of the local wind effects in simulations had worsened the results. However, the form of the vertical profiles with wave effects (Runs 3 and 6) was good in comparison to measurements except for near



the seabed, showing that the discrepancies came from the bottom friction, which appeared as being mis-evaluated.

For RTF1 and RTF3, the results with and without waves (Runs 3, 6 10; see Figure 12) were quite similar and fitted the ADCP data. Wave effects had little improved the results by reducing the velocity, except for Run 6 at RFT3. At these time points, as the tidal current was higher at about 1.9 m/s at the surface (see Table 5) and the waves were small (see Figure 6a), the wave effects were overwhelmed by the tidal effects and particularly those ascribed to Stokes drift, which were in the order a few cm/s. At RTE3, as explained previously, the tidal current was abnormally reduced.

During the ebb, at RTE1, RTE2, RTE3 and RTE4,  $|\Delta\theta_{w/c}|$  were less than  $90^\circ$  and around  $60^\circ$ . For all time points, waves effects had improved the results by reducing the velocity in the upper water column due Stokes drift effects, as expected in the former studies. Nice fits with observations were obtained, particularly at RTE3 and RTE4 where waves are more energetic than at RTE1 and RTE2 (see Figure 6). Local wind effects, which had induced error during the flood on 25 November, had not worsened the vertical structure at RTE3 and RTE4.

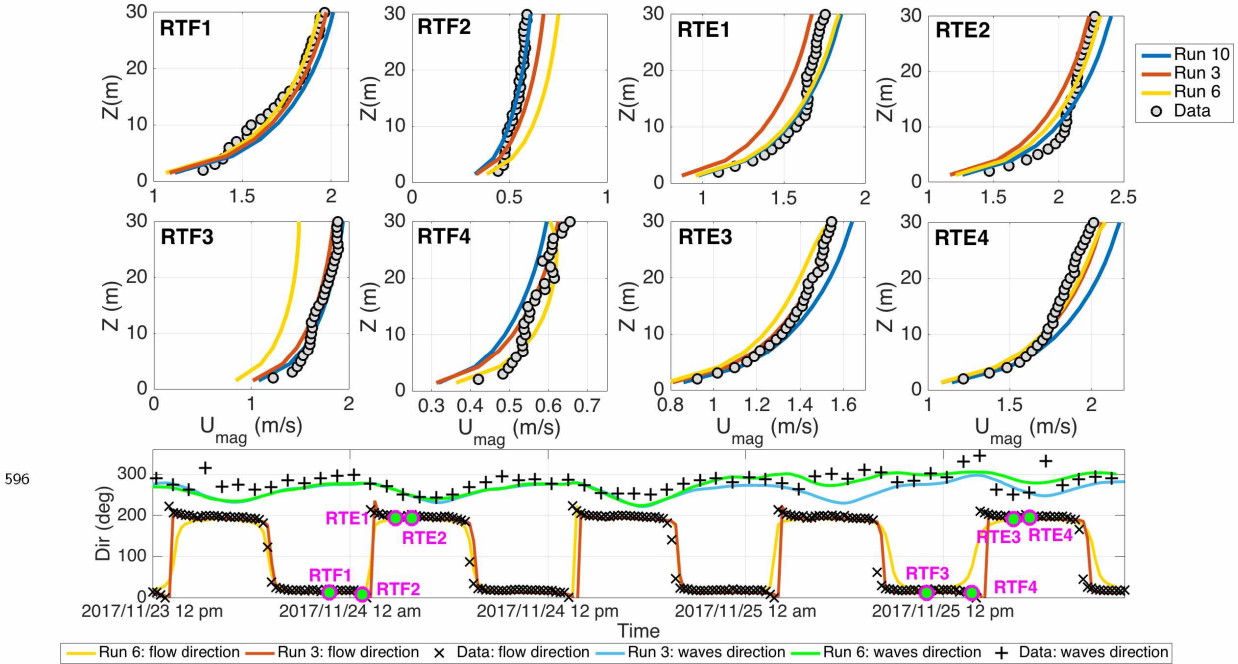


FIGURE 12: **Top and middle panels** : Flow velocity magnitude over depth at RTF1, RTF2, RTE1, RTE2, RTF3, RTF4, RTE3 and RTE4; in-situ measurements are in black circles while numerical results for Runs 3, 6 and 10 are in red, yellow and blue lines, respectively. **Bottom panel** : Time series of the flow direction for Run 3 (red line) and 6 (yellow line) and the related wave direction in light blue and green line, respectively. Measured wave and current directions are in black crosses and plusses.

## 4. Conclusions and perspectives

The purpose of this study is twofold with a first step dealing with the validating of our modelling platform for the study site and a second step aiming to show the impacts of wave-current interactions on the hydrodynamic of the Alderney Race. To reach these goals, realistic 3D fully-coupled wave-current-turbulence simulations have been carried out and tested against in-situ measurements.

On the whole, our numerical model is successfully validated through statistical parameters (PBIAS, NRMSE, MAE,  $R^2$ ) in comparison with observations for mean sea water level, significant wave height, mean wave direction, frequency wave energy spectra, flow velocity magnitude and direction. However, a non-stationary time lag was observed sometimes between model and measurements. This problem was found to be sensitive to the waves and wind effects and had been partially fixed when these effects were included, probably due to the near-bed wave orbital velocity which changes the bottom stress. In addition, time lag was also shown as being modified by the depth where a logarithmic velocity profile is applied, highlighting the effects of the near-bed turbulence. Therefore, further studies are required to investigate what are the role in the time lag of the bottom turbulence, near-bed wave orbital velocity as well as the bathymetry effects, that drive the hydrodynamic. Furthermore, when the wind blowed hard (wind speed greater than 15 m/s), the flow velocity was abnormally decreased (of about 0.5 m/s) due to a mis-evaluating of the local wind effects in the ocean model. This point needs to be improved in the future by working on the wind stress formulation and the relating wave contribution.

Wave-current interactions were observed in Alderney Race. Ocean waves impacted the flow due to : - the Stokes drift effects, that induced an increase/decrease in the current depending on the angle between waves and current, with a maximum influence near the surface, - the wave enhancement of the bottom friction that reduced the tidal current. Furthermore, tidal current has modified ocean waves through : - the refraction of waves by the current, that have generated changes in waves directions and - the wave breaking ascribed to tidal current, that increased the turbulent mixing within the water column. The main results of this paper is the significant influence of ocean waves on the vertical profile of the flow whereas waves are small (significant wave height less than 1.5 m). Moreover, changes in vertical profiles were occurred even for a strong surface current (around 2.3 m/s) due to the angle between waves and flow direction.

Consideration of ocean waves effects has improved the simulation of the tidal current and particularly the reproduction of its vertical shape, showing that these effects have to be taken into account for the tidal converter dimensionning.

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### **Declaration of interests**

X The authors declare that they have no known competing financial interests or personal relationships that could have appeared to influence the work reported in this paper.

☐ The authors declare the following financial interests/personal relationships which may be considered as potential competing interests:

On behalf of the authors

Anne-Claire Bennis

## **Author Contribution Statement**

**A.-C. Bennis:** Conceptualization, Writing – original draft, Writing – review and editing, Methodology, Software, Validation, Investigation, Supervision, Funding Acquisition.

**L. Furgerot:** Writing – review and editing, Data curation, Formal analysis, Validation, Resources.

**P. Bailly du Bois:** Writing – review and editing, Investigation.

**F. Dumas:** Writing – review and editing, Investigation.

**T. Odaka:** Writing – review and editing, Software.

**C. Lathuilière:** Writing – review and editing, Resources.

**J.-F. Filipot:** Writing – review and editing, Funding Acquisition.